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# The importance of calibration parameters on the accuracy of the floods description in the Snyder's model

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30-059 Kraków, Poland; e-mail: a.walega@ur.krakow.pl**For citation:** Wałęga A. 2016. The importance of calibration parameters on the accuracy of the floods description in the Snyder's model. Journal of Water and Land Development. No. 28 p. 19–25.

## Abstract

Comparison of quality of Snyder's model for determination flood waves was examination in this work. Model parameters were calibrated based on objective functions: percentage error in peak flow (*PEPF*), percentage error in volume (*PEV*), peak-weighted root mean square error (*PWRMSE*), sum of absolute residuals (*SAR*) and sum of squared residuals (*SSR*). Quality of model was calculating by Nash-Sutcliffe efficiency coefficient *E*. Additionally sensitivity of a model was characterized by its flexibility. The analyses were performed in the watershed of Grabinka. It has been found that the use *PWRMSE* as the objective function allows to obtain the best quality results of simulation. Furthermore, Snyder's model is flexible to the change of  $C_p$  coefficient.

**Key words:** *efficiency of model, flexibility of model, objective function, unit hydrograph*

## INTRODUCTION

Estimating an appropriate design value for extreme flood events such as maximum water levels, discharges, or runoff is one of the most important tasks of the design process for a large number of engineering projects and studies that reduce the impact of flooding [GADEK 2014; KARABOVÁ *et al.* 2012; VAŠŠOVÁ 2013]. Unfortunately, in many streams are ungauged and do not have flow records. Even when stream gauges are in place, the record is often too short to accurately predict extreme events [KHALEGHI *et al.* 2014]. In this case, for the estimation of design floods and volumes, often, event-based hydrological models are applied. Simulation models of watershed hydrology are extensively used for water resources planning and management. These models can offer a sound scientific framework for watershed analyses of water movement and provide reliable information on the behavior of the system [JHA 2011]. In practice hydrologists uses a synthetic unit hydrograph to transformation of effective rainfall to runoff. A synthetic

unit hydrograph is a unit hydrograph derived following an established formula, without the need the rainfall-runoff data analysis [WAŁĘGA 2013; WAŁĘGA *et al.* 2011]. This includes Snyder's method, Soil Conservation Service (SCS) method, Gray's method and Clark's instantaneous unit hydrograph method. Understanding the hydrology of a watershed and modeling different hydrological processes within a watershed are therefore very important for assessing the environmental and economical well-being of the watershed. Besides successful application of hydrological models, there are several issues that question the model output such as uncertainty in input parameters, nonlinear relationships between hydrologic input features and hydrologic response, and the required calibration of numerous model parameters. These issues can be examined with sensitivity analyses of the model parameters to identify sensitive parameters with respect to their impact on model outputs. Proper attention to the sensitive parameters may lead to a better understanding and to better estimated values and thus to reduced uncertainty [LENHART *et al.* 2002]. The



the observed rainfall-discharge phenomenon. The Snyder's model based on synthetic unit hydrographs was subjected to the calibration process.

In 1938 Snyder introduced the concept of a synthetic unit hydrograph. Based on the analysis of a large number of hydrographs for watersheds in the Appalachian region the author derived the following equation for the lag time:

$$T_{lag} = 0.75C_t(LL_c)^{0.3} \quad (1)$$

where:  $T_{lag}$  = lag time, hours;  $C_t$  = coefficient dependent on the watershed slope and related to watershed retention;  $L$  = distance along the main stream, from the estuary section to crossing of dry valley with water divide, km;  $L_c$  = distance along the main stream from the estuary section to the watershed gravity center, km.

The flow in the culmination is calculated from the following equation:

$$Q_p = \frac{0.275C_p \cdot A}{T_{lag}} \quad (2)$$

where:  $Q_p$  = flow in the culmination of unit hydrograph caused by effective precipitation of 1 cm height,  $m^3 \cdot s^{-1}$ ;  $A$  = watershed area,  $km^2$ ;  $C_p$  = empirical coefficient resulting from simplifying the shape of the hydrograph into the triangle [PONCE 1989].

Time to peak is calculated from the equation:

$$T_p = \frac{D}{2} + T_{lag} \quad (3)$$

where:  $T_p$  = time to peak, hours;  $T_{lag}$  = lag time, hours;  $D$  = duration of effective precipitation, hours.

In the first stage of the calculations, model parameters were automatically calibrated based on the following measures (objective functions): percentage error in peak flow (*PEPF*), percentage error in volume (*PEV*), peak-weighted root mean square error (*PWRMSE*), sum of absolute residuals (*SAR*) and sum of squared residuals (*SSR*). All characteristics are specified with the equations [CUNDERLIK, SIMONOVIC 2004]:

$$PEPF = 100 \left[ \frac{Q_o - Q_s}{Q_o} \right] \quad (4)$$

$$PEV = 100 \left[ \frac{V_o - V_s}{V_o} \right] \quad (5)$$

$$PWRMSE = \sqrt{\frac{\sum_{t=1}^N (Q_{ot} - Q_{st})^2 \cdot \frac{Q_{ot} + Q_{ave}}{2Q_{ave}}}{N}} \quad (6)$$

$$Q_{ave} = \frac{1}{N} \sum_{t=1}^N Q_{ot}$$

$$SAR = \sum_{t=1}^N |Q_{ot} - Q_{st}| \quad (7)$$

$$SSR = \sum_{t=1}^N [Q_{ot} - Q_{st}]^2 \quad (8)$$

where:  $Q_{ot}$  and  $Q_{st}$  = observed and simulated flow in time  $t$ ;  $Q_{ave}$  = average observed flow;  $V_o$  and  $V_s$  = volumes of observed and simulated wave.

Automatic calibration of model parameters was based on iterative selection of the parameters until the minimum of the objective function. In order to minimize the objective function (4) to (8) a uniform gradient method was applied. This involves estimating the value of one parameter while maintaining the remaining stable.

The final evaluation of the calibration process was based on Nash-Sutcliffe efficiency coefficient  $E$  [NASH, SUTCLIFFE 1970], commonly used in hydrology:

$$E = \left[ 1 - \frac{\sum_{i=1}^N (Q_{oi} - Q_{si})^2}{\sum_{i=1}^N (Q_{oi} - \bar{Q}_o)^2} \right] \quad (9)$$

where:  $N$  = number of hydrograph ordinates,  $i$  = index changing from 1 to  $N$ ,  $Q_{oi}$  =  $i^{\text{th}}$  ordinate of the observed hydrograph,  $Q_{si}$  =  $i^{\text{th}}$  ordinate of the simulated hydrograph,  $\bar{Q}_o$  = average of the observed hydrograph ordinates.

Sensitivity of a model was characterized by its flexibility, which is a measure of impact of one parameter on another. It is a non-unitary parameter, which is calculated as the ratio of the percentage change in the output characteristics to the percentage change in input parameter [MAIDMENT, HOOGERWERF 2002; WAŁĘGA 2014]. If values of this parameter are higher or equal to 1, then the parameter is "flexible", in other words, the dependent variable is very sensitive to the size of independent variable. Otherwise, when flexibility is lower than 1, the parameter is "inflexible" and the dependent variable is not sensitive to the change of independent variable. The aim of the sensitivity analysis was to determine the effect of time lag and the  $C_p$  coefficient in Snyder's model on the variability of culmination flow in a simulated hydrograph. The analysis consisted in setting different values of parameters and calculating  $Q_{max}$  flow.

## RESULTS AND DISCUSSION

In the first stage of the analysis the general characteristics of selected flood waves was conducted, which is presented in Table 1. Three out of four of the analyzed waves happened in the summer half-year (waves from 1980, 2004 and 2006), while the wave from 2006 occurred in late May and early June. The mentioned wave from 2006 was characterized by the highest culmination, which reached  $43.7 m^3 \cdot s^{-1}$ . The wave from the summer half-year of 2004 was characterized by the longest duration – 480 h. The duration of other waves was similar and did not exceed 240 h. The culmination time was from 24 to 72 h but the longest time occurred in the wave which was characterized by the lowest culmination among the analyzed

**Table 1.** Characteristics of the analyzed flood waves

Date of wave occur	$Q_{max}$	$Q_b$	$T_b$	$T_w$	$T_o$	$V_c$	$V_w$	$V_o$	$\alpha$
08.1980	37.50	1.49	240	24	216	7.85	3.37	4.46	9.00
03.1981	25.00	2.92	216	24	192	5.45	2.41	3.04	8.00
07–08. 2004	18.00	0.61	480	72	408	5.86	2.04	3.82	5.70
05–06. 2006	43.70	1.85	204	36	156	7.71	4.06	3.65	4.30

Explanations:  $Q_{max}$  = flow at culmination,  $m^3 \cdot s^{-1}$ ;  $Q_b$  = flow at wave base,  $m^3 \cdot s^{-1}$ ;  $T_b$  = duration of the wave, h;  $T_w$  = time to culmination, h;  $T_o$  = descending time, h;  $V_c$  = wave volume,  $10^6 m^3$ ;  $V_w$  = volume of ascending part of the wave,  $10^6 m^3$ ;  $V_o$  = volume of the descending part of the wave,  $10^6 m^3$ ;  $\alpha$  = coefficient of wave slenderness.  
 Source: own study.

waves. The wave with the highest culmination of  $43.7 m^3 \cdot s^{-1}$  was caused by the precipitation of 107.3 mm, while paradoxically the wave of the lowest culmination –  $18.0 m^3 \cdot s^{-1}$  was the result of much higher precipitation – 124.2 mm. This can be explained by the fact that the wave at the turn of May–June 2006 was

preceded by a long-term precipitation (6-day), which resulted in ground soaking. After this period, precipitation occurred on June 3<sup>rd</sup> with a height of 59 mm per day, which caused a distinct culmination – Fig. 2a. Undoubtedly, the culmination of the discussed wave could have been impacted by high groundwater table after a spring snowmelt period. In the case of an episode of July–August 2004, precipitation fell on over-dried ground, so despite substantial amount of water, the watershed was characterized by a significant water retention capacity, which greatly reduced the wave culmination – Fig. 2b. The two major waves from 1980 and 2006 were characterized by the largest total volume. Coefficient of slenderness of flood wave  $\alpha$ , calculated according to CIEPIEŁOWSKI [1987] as the relation of wave falling time to the culmination time, indicates the fact that the waves from 1980 and 1981 increased very quickly and then the water level slowly descended, whereas the other two waves had slightly smaller imbalance between the descend and increase time.

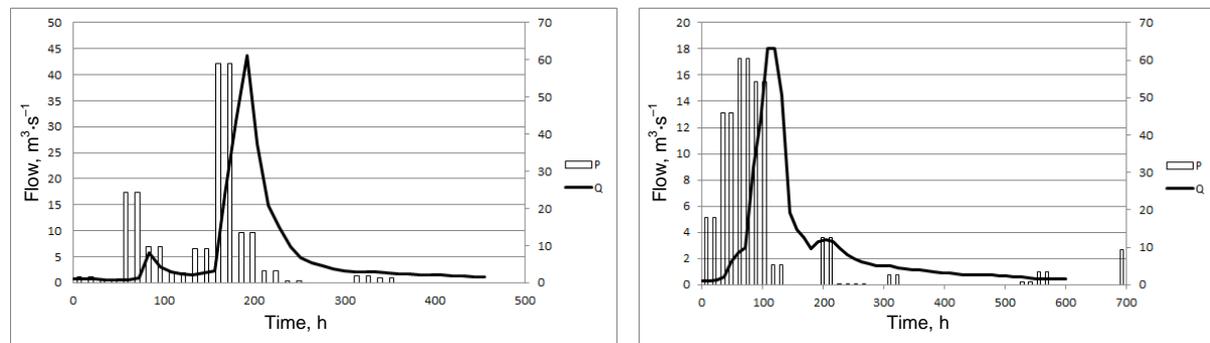


Fig. 2. The course of precipitation and resulting floods in the case of episode a) from 2006 and b) from 2004; source: own study

Table 2 lists the statistical measures of the objective functions for the analyzed waves predicted by Snyder’s unit hydrograph.

**Table 2.** The values of the objective functions described by Snyder’s model

Wave	PEPF	PEV	PWRMSE	SAR $m^3 \cdot s^{-1}$	SSR
	%				
1980	0.0 (0.0)	0.0 (-20.4)	3.30 (-6.40)	28.9 (-10.8)	230.2 (-21.9)
1981	0.0 (0.0)	0.0 (-43.0)	4.50 (-18.2)	50.5 (-25.7)	342.3 (-28.6)
2004	0.0 (0.0)	78.1 (-10.8)	2.80 (6.4)	131.6 (63.4)	756.6 (18.3)
2006	0.0 (0.0)	0.0 (-74.9)	5.8 (-19.8)	74.9 (-41.5)	814.8 (-42.3)

Explanations: PEPF = percentage error in peak flow, PEV = percentage error in volume; PWRMSE = peak-weighted root mean square error, SAR = sum of absolute residuals, SSR = sum of squared residuals, in branches are percentage different between peak flows in observed and simulated wave.  
 Source: own study.

Calculations have shown, that by using the objective function described by formula (4) one can obtain complete consistency between culminations of calcu-

lated and observed waves – Table 2. A similar principle shall apply in the case of the other measure – PEV, except that in this case, the model tends to minimize differences in the volume between the compared waves. Unfortunately, adopting these criteria may have negative impact on the shape of the calculated wave. This is presented in Figure 3, which compares the results of wave simulation from 1980, described by Snyder’s model using the PEPF and PWRMSE objective functions.

It should be noted, that the best calibration results were obtained using PWRMSE. Although the differences between flows in culminations and flows observed in PWRMSE are higher than in the one described by the formula (4), the shape of the calculated hydrograph much better describes the entire wave, compared to PEPF – Figure 3. Apart from the PEPF criterion, the best calibration results for each of the analyzed waves were obtained based on PWRMSE – Table 2. Flow culminations were slightly underestimated in three out of four analyzed cases. The best calibration results obtained from the PWRMSE model come from the fact that in the optimization procedure the errors of flows located closer to the culmination

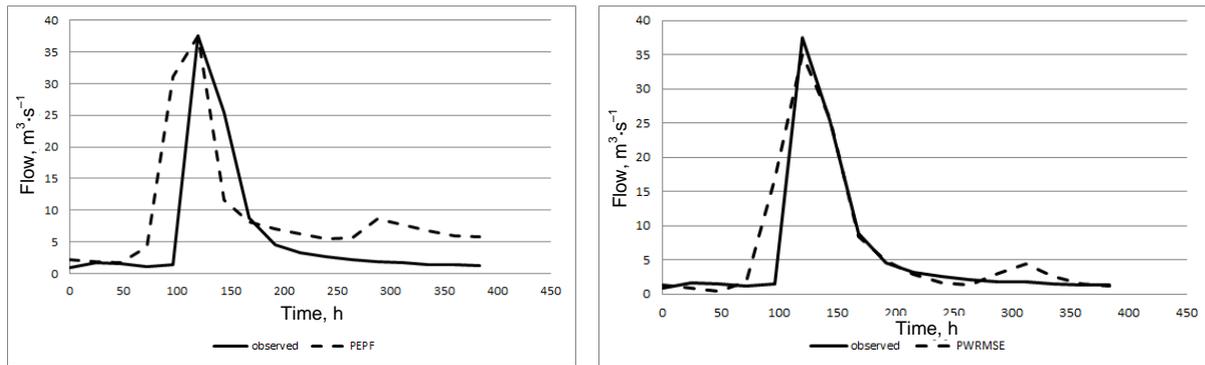


Fig. 3. Comparison of the observed wave from 1980 with the calculated Snyder's model using the objective functions: a) percentage error in peak flow (PEPF) and b) peak-weighted root mean square error (PWRMSE); source: own study

flows are assigned higher weights. In the case of SAR measure, the error weights are independent of their errors, while for SSR measure the weights are higher for high error values and lower for low values. The smallest differences between  $Q_{max}$  were obtained for

the wave from 2004 and the highest – for the wave from 2006. Table 3 presented values of parameters of Snyder's model. Parameters of Snyder's model have large variation for analyzed waves and for different objective functions.

Table 3. The values of parameters of the Snyder's model for different objective functions

Wave	PEPF			PEV			PWRMSE			SAR			SSR		
	$C_p$	$T_{lag}$ h	$T_p$ h	$C_p$	$T_{lag}$ h	$T_p$ h	$C_p$	$T_{lag}$ h	$T_p$ h	$C_p$	$T_{lag}$ h	$T_p$ h	$C_p$	$T_{lag}$ h	$T_p$ h
1980	0.51	19.5	48	0.65	26.5	48	0.78	33.9	48	0.78	34.8	48	0.78	34.9	48
1981	0.52	19.8	48	0.73	44.0	72	0.53	20.1	48	0.52	19.9	48	0.65	26.7	48
2004	0.43	36.5	96	0.40	45.0	132	0.80	30.0	96	0.50	30.1	60	0.40	30.4	60
2006	0.80	78.4	120	0.40	120.2	120	0.77	53.5	60	0.78	51.6	60	0.59	52.0	60

Explanations: PEPF, PEV, PWRMSE, SAR, SSR as under Table 2,  $C_p$  = empirical coefficient,  $T_{lag}$  – lag time,  $T_p$  = time to peak. Source: own study.

The next stage of the calibration of model parameters is the general assessment of the quality, which was performed using the coefficient of efficiency  $E$ . The results are presented in Table 4.

Table 4. Values of the coefficient of efficiency  $E$  (%) for each objective function

Wave	Snyder's unit hydrograph				
	PEPF	PEV	PWRMSE	SAR	SSR
1980	22.3	75.7	84.0	86.7	85.8
1981	-3.1	22.4	23.6	11.8	36.2
2004	31.4	33.1	80.7	-30.2	13.6
2006	-46.5	-5.1	74.5	81.4	75.1

Source: own study.

The lowest efficiency of both models was obtained when their parameters were optimized using PEV and PEPF functions. The reason for this, as previously mentioned, was that the model parameters were selected so that the consistency between the observed and calculated wave occurred only between  $Q_{max}$  and  $V$  without considering the shape of hydrographs. Generally, when comparing both objective functions, slightly better results were obtained by optimizing the model parameters using PEV. Approximated values of the coefficient of efficiency  $E$  were obtained when optimization of model parameters was performed using the other objective functions. This

follows from the fact that the error values are calculated for the entire course of the hydrograph and not for its selected parts. When adopting the correctness criteria for the description of reality by the model given in the paper by MORIASI *et al.* [2007], who stated that the calibration process may be considered successfully completed when the value of Nash-Sutcliffe coefficient of efficiency is above 65%, it can be concluded, that in the case of Snyder's model and parameter calibration using PWRMSE, this model can be considered correct in 3 out of 4 analyzed waves. The lowest value of the coefficient  $E$  was obtained for the wave of 1981 and this resulted from the fact that the model imprecisely approximated the second culmination in this episode. In the case of SAR and SSR Snyder's model correctly described the two waves – from 1981 and 2006.

The final element of the model parameter calibration was the flexibility analysis. In the case of Snyder's model the impact of time lag  $T_{lag}$  and flow coefficient  $C_p$  on the flow values in culmination was analyzed. The calculations were performed for varying values of  $T_{lag}$  in 5 min interval and  $C_p$  of 0.1 intervals. The calculations revealed that in the case of  $T_{lag}$  the model is inflexible. The calculated flexibility ratio for  $T_{lag}$  equaled -0.33. The change of  $T_{lag}$  primarily postpones the hydrograph, while reducing the  $Q_{max}$  is observed mostly for low time lag values. For higher

$T_{lag}$  values (over 30 h) the results stabilize – Figure 4a. Time lag values are strongly correlated with the watershed slope, its shape and retention properties [BELETE 2009; SALAMI *et al.* 2009]. The model is flexible for the changes of  $C_p$  parameter (the coefficient of efficiency value was 0.99). Increase of the  $C_p$

parameter indicates the reduction of watershed retention capacity, which contributes to significant increase in the size of flows – Fig. 4b. With an increase of  $C_p$  value by 0.6, the flow  $Q_{max}$  was over 4-times higher. The hydrograph postponing was not observed.

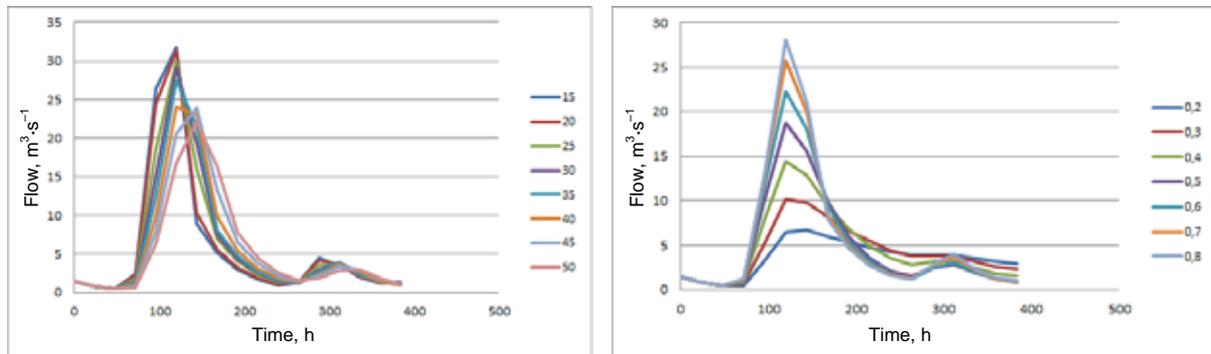


Fig. 4. Effect of a change of: a) lag time ( $T_{lag}$ ) and b) empirical coefficient ( $C_p$ ) parameter, on the shape of discharge hydrograph in Snyder's model; source: own study

## CONCLUSIONS

1. The smallest differences between the maximum flow in the culmination of observed and calculated waves were obtained using *PWRMSE*. This follows from the fact that in the optimization procedure higher weights were assigned to the errors of flows located closer to the culmination flows.

2. Parameters of Snyder's model have large variability for different objective function.

3. Snyder's model is flexible to the change of  $C_p$  coefficient. Increase of this parameter's value reduces the retention ability of a watershed and significantly affects the increasing flows without changing the shape of the hydrograph.

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**Andrzej WAŁĘGA**

**Znaczenie procesu kalibracji parametrów dla poprawności opisu wezbrań w modelu Snydera**

STRESZCZENIE

**Słowa kluczowe:** efektywność modelu, elastyczność modelu, hydrogram jednostkowy

W pracy porównano efektywność modelu Snydera w opisie wezbrań opadowych, którego parametry szacowano za pomocą różnych funkcji celu: procentowego błędu przepływu w kulminacji – *PEPF*, procentowego błędu objętości fali – *PEV*, ważonych średnich kwadratów błędu – *PWRMSE*, absolutnych sum reszt – *SER* i sum kwadratów reszt – *SSR*. Jakość modelu oceniono za pomocą współczynnika efektywności Nash–Sutcliffe’a. Dodatkowo zbadano wpływ zmiany wartości parametrów na wielkości przepływu uzyskane z modelu za pomocą współczynnika elastyczności. Analizy prowadzono w zlewni rzeki Grabinki. Stwierdzono, że zastosowanie *PWRMSE* jako funkcji celu umożliwi na uzyskanie najlepszej zgodności wyników z modelu z przepływami obserwowanymi. Ponadto model Snydera jest wrażliwy na zmiany parametru  $C_p$ , który może być utożsamiany z retencyjnością zlewni.